



ADDENDUM #1

Tuesday, April 26, 2022

**JOB # BC2021-08-B
RENOVATIONS & ADDITIONS TO BAYOU CHICOT ELEMENTARY SCHOOL
EVANGELINE PARISH SCHOOL BOARD - OWNER**

Each bidder shall note on his Bid Form receipt of Addendum #1; that his Bid is for the conditions set forth in this Addendum plus the complete Contract Documents.

SOLE RESPONSIBILITY FOR PROPER DELIVERY OF BID IN COMPLIANCE WITH THE OFFICIAL PROJECT ADVERTISEMENT IS THAT OF THE GENERAL CONTRACTOR'S.

PROJECT MANUAL

CORRECTION: This Project is to be completed in 365 (three hundred sixty-five) days.

CLARIFICATION: The liquidated damages if any there be, shall be set at \$200 (two hundred dollars) per day.

ADDITION: Soil Report generated by Geotechnical Testing Laboratory, Inc.

ADDITION: Drainage Basin Schedule

ARCHITECTURAL DRAWINGS : A101 OVERALL SITE PLAN : PROVIDE ONE 10'-0" WIDE SIDEWALK TRENCH DRAIN WHERE NEW WALKWAY TIES INTO EXISTING BEHIND (west) of BUILDING C.

THE FOLLOWING DOCUMENTS ARE ATTACHED TO THIS ADDENDUM AND ARE HEREBY MADE A PART OF THIS ADDENDUM #1 (ONE) :

- NEW BID FORM WITH APPENDIX A**
- SOIL REPORT**
- DRAINAGE BASIN SCHEDULE**

END OF ADDENDUM #1 (ONE)

LOUISIANA UNIFORM PUBLIC WORK BID FORM

TO: **Wayne Dardeau, President**
Evangeline Parish School Board
1123 Te Mamou Rd.
Ville Platte, LA 70586

BID FOR: **Job# BC2021-08-B**
Renovations and Additions to
Bayou Chicot Elementary School

The undersigned bidder hereby declares and represents that she/he; a) has carefully examined and understands the Bidding Documents, b) has not received, relied on, or based his bid on any verbal instructions contrary to the Bidding Documents or any addenda, c) has personally inspected and is familiar with the project site, and hereby proposes to provide all labor, materials, tools, appliances and facilities as required to perform, in a workmanlike manner, all work and services for the construction and completion of the referenced project, all in strict accordance with the Bidding Documents prepared by: **Braddock Companies, LLC** and dated: **March 2022.**

Bidders must acknowledge all addenda. The Bidder acknowledges receipt of the following **ADDENDA:** (Enter the number the Designer has assigned to each of the addenda that the Bidder is acknowledging) _____.

TOTAL BASE BID: For all work required by the Bidding Documents (including any and all unit prices designated "Base Bid" * but not alternates) the sum of:

_____ Dollars (\$ _____)

ALTERNATES: For any and all work required by the Bidding Documents for Alternates including any and all unit prices designated as alternates in the unit price description.

Alternate No. 1 *Refinishing of Gymnasium Floor* for the lump sum of:
_____ Dollars (\$ _____)

Alternate No. 2 *(Not Applicable)* for the lump sum of: *(Not Applicable)*
_____ Dollars (\$ _____)

Alternate No. 3 *(Not Applicable.)* for the lump sum of: *(Not Applicable)*
_____ Dollars (\$ _____)

NAME OF BIDDER: _____

ADDRESS OF BIDDER: _____

LOUISIANA CONTRACTOR'S LICENSE NUMBER: _____

NAME OF AUTHORIZED SIGNATORY OF BIDDER: _____

TITLE OF AUTHORIZED SIGNATORY OF BIDDER: _____

SIGNATURE OF AUTHORIZED SIGNATORY OF BIDDER **: _____

DATE: _____

* The Unit Price Form shall be used if the contract includes unit prices. Otherwise it is not required and need not be included with the form. The number of unit prices that may be included is not limited and additional sheets may be included if needed.

** If someone other than a corporate officer signs for the Bidder/Contractor, a copy of a corporate resolution or other signature authorization shall be required for submission of bid. Failure to include a copy of the appropriate signature authorization, if required, may result in the rejection of the bid unless bidder has complied with La. R.S. 38:2212(B)5.

BID SECURITY in the form of a bid bond, certified check or cashier's check as prescribed by LA RS 38:2218.A is attached to and made a part of this Bid.

APPENDIX A is attached hereto and made a part of this Bid, and shall be executed and submitted with the Bid Form.

BID FORM APPENDIX A

BID DATE: 2:00 P.M., MAY 12, 2022

COMPLETION TIME: The Bidder hereby agrees to compete the Contract within **three hundred sixty-five (365) days** from the date of the Notice to Proceed, or within the time as may be extended by Change Order. The Work shall be considered complete only when the Owner shall have filed with the Office of the Recorder of Mortgages for the Parish in which the Work is located, its formal certificate of acceptance of the Work.

LIQUIDATED DAMAGES: The Bidder hereby also agrees, as a part consideration for this Contract, to pay the following sums per calendar day of Liquidated Damages, for such breach of Contract, if the Work is not completed within stipulated days: Two Hundred Dollars (\$200.00) per calendar day.

REJECTION OF BIDS: The Bidder understands that the Owner reserves the right to reject any or all bids and waive any informalities in the bidding.

WITHDRAWAL OF BIDS: The Bidder agrees that this Bid shall be good and may not be withdrawn for a period of thirty (30) calendar days after the scheduled closing for receiving bids except in accordance with the provisions of Act 111 of 1983. This Bid may be withdrawn at any time prior to the scheduled time for the opening of bids or any postponement thereof.

PERFORMANCE BOND: The Bidder hereby certifies that he has included in his proposal a sum sufficient to defray the cost of a 100% Performance and Material Payment Bond required by the Contract Documents, if declared the successful Bidder.

AFFIDAVIT: That the Owner comply with Legislative Act No. 38, State of Louisiana, it is understood that all contractors receiving value for services rendered for construction of this building shall execute an Affidavit attesting that said Public Contract was not secured through employment or payment of any solicitor.

LICENSE CERTIFICATION: The Bidder certifies that he/she meets all licensing requirements of this State and is duly and currently licensed under R.S. 37:2151-2163 of the State of Louisiana.

SIGNED and AGREED to on this _____ day of _____, 2022.

BIDDER: _____

BY: _____
(signature)

(printed)



Geotechnical Testing Laboratory, Inc.

Engineering and Construction Materials Testing Services

March 4, 2022

Braddock Companies, LLC
4024 Jackson Street
Alexandria, Louisiana 71303

Attention: Mr. Mike Holt, AIA

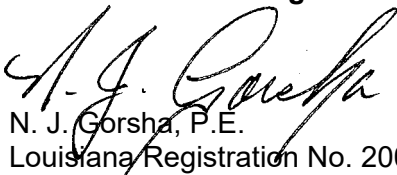
**RE: Geotechnical Investigation Services
A New Multi-Purpose Building for
Bayou Chicot Elementary School
Ville Platte, Evangeline Parish, Louisiana
Job Number BC2021-08-B
Report Number 03-22-036**

Dear Mr. Holt:

Geotechnical Testing Laboratory, Inc. is pleased to submit this report of subsurface exploration for the above referenced project. Included in the report are the results of the exploration and recommendations concerning the design and construction of the foundations as well as general site development.

We appreciate the opportunity to have provided you with our geotechnical engineering services. If you have any questions concerning this report, or if we may be of further service, please contact our office.

Respectfully submitted,
Geotechnical Testing Laboratory, Inc.


N. J. Gorsha, P.E.
Louisiana Registration No. 20082


Ken Gorsha
President

Distribution: (3) Braddock Companies, LLC

NJG/krq



Geotechnical Investigation Services
**A New Multi-Purpose Building for
Bayou Chicot Elementary School**
Ville Platte, Evangeline Parish, Louisiana
Job Number BC2021-08-B
Report Number 03-22-036

Prepared For:

Braddock Companies, LLC
4024 Jackson Street
Alexandria, Louisiana 71303

Prepared By:

Geotechnical Testing Laboratory, Inc.
226 Parkwood Drive
Alexandria, Louisiana 71301

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Geotechnical Investigation Services
**A New Multi-Purpose Building for
Bayou Chicot Elementary School**
Ville Platte, Evangeline Parish, Louisiana
Job Number BC2021-08-B
Report Number 03-22-036

Introduction:

This report transmits the findings of a geotechnical investigation performed for the above-referenced project. The purpose of this investigation was to define and evaluate the general subsurface conditions in the immediate vicinity of a proposed new facility. Specifically, the study was planned to determine the following:

- Subsurface stratigraphy within the limits of our exploratory borings.
- Classification, strength, and compressibility characteristics of the foundation strata.
- Suitable foundation systems and allowable soil bearing pressures.
- Construction requirements for the placement of select earth fills.

The purpose of this report is to provide the architect, structural engineer, civil engineer, and other design team professionals with recommendations for the design and construction of the proposed project. This report should not be used by the contractor in lieu of project plans or specifications.

Project Authorization:

Formal authorization to perform the work was provided by Mr. Mike Holt, AIA with Braddock Companies, LLC(Client), by accepting our February 24, 2021 written proposal. Authorization to proceed was provided in an electronic mail on the same day. Field procedures were conducted on March 1, 2021. To accomplish the intended purposes, a three-phase study program was conducted which included:

- a field investigation consisting of four exploratory test borings with samples obtained at selected intervals;
- a lab testing program designed to evaluate the expansive and strength characteristics of the subsurface soils; and,
- an engineering analysis of the field and laboratory test data for foundation design recommendations.

No additional analysis was requested. A brief description of the field and laboratory test procedures are provided in the Appendix.

Project Description:

We understand the project will consist of a one-story, slab-on-grade, steel-frame multi-purpose building containing approximately 8,959 square feet of area. A structure with a deep foundation system consisting of drilled cast-in-place concrete shafts formerly occupied the site. Prior to our field exploration, the tops of the shafts were removed approximately 36 to 48 inches below grade, and the resulting excavations were backfilled with soil. A common floor level is planned throughout. No below grade walls are anticipated.

Maximum column and wall loads will reportedly not exceed approximately 50 kips (1 kip = 1,000 pounds) and 1.5 kips per linear foot, respectively. Based on the existing site topography, it

appears that the site is essentially, or very near, finish grade. If any grade changes are anticipated, these should be discussed with our geotechnical engineer prior to finalizing design.

If any of this information should change significantly or be in error, it should be brought to our attention so that we may review recommendations made in this report.

Site Conditions:

The project site is located west of US Highway 167 at Bayou Chicot Elementary School in Ville Platte, Evangeline Parish, Louisiana. The site was noted to be relatively level with estimated maximum elevation differences of less than one (1) foot, and void of vegetation at the time of drilling. The drilling rig experienced moderate difficulty moving about the site.

Subsurface Stratigraphy:

The subsurface conditions at the proposed building site were explored by drilling a total of four (4) borings to a depth of approximately 20 feet. The borings were located in the field by the drilling crew as shown on the Plan of Borings included in the Appendix of this report.

The stratification of the soils encountered during field drilling operations is presented on the boring logs in the Appendix. The stratification of the subsurface materials shown on the boring logs represents the subsurface conditions encountered at the actual boring locations and variations may occur across the site. The lines of demarcation represent the approximate boundary between the soil types, but the actual transition may be gradual. The following subsurface descriptions are of a generalized nature to highlight the major stratification features. The boring logs should be reviewed for more detailed information.

In order of increasing depth, the borings generally encountered the following soil strata beneath the surface: lean clay (CL)s, (CL), poorly graded sand (SP-SM) and silty sand (SM).

Groundwater Conditions:

Groundwater seepage was not observed during advancement of the test borings and, after short time lapses, the borings remained dry and un-caved. The subsurface water regime is always subject to change with variations in climatic conditions and will likely coincide seasonal fluctuations. Future construction activities may also alter the surface and/or subsurface drainage patterns of this site. Therefore, groundwater conditions should be explored at the start of construction by others due to short-term observations by our field crew.

Perched water may be briefly encountered in low quantities during earthwork and is typically due to storage of recent rainfall or by a barrier to capillary evaporation. Where perched water is encountered, the contractor should expect to excavate gravity drainage ditches to divert it away from the construction area. The depth of the ditches should be at least two (2) to three (3) feet deeper than the lowest exterior footing elevation. Additionally, soft, wet and pumpable soils can be expected below perched water tables. In structural areas, these should be removed to firm ground and replaced with select fill soils compacted to project specifications as defined later in this report.

Foundation Recommendations:

Detailed information on structural systems and planned grading was not available to us at the time this report was prepared. Based on the size and type of structure, as well as the findings from this investigation, a system of shallow footings with an on-grade floor slab, in conjunction with the recommended subgrade preparation is believed to be the most practical and economical means of support.

A Potential Vertical Rise (PVR) value of less than one (1) inch was determined for this site. One (1) inch of PVR is generally accepted as the maximum allowable value for design and construction in the geographical area. The surficial soils encountered by the borings are considered to be moderately expansive. There should be no requirement for the removal of swelling soils at this site.

Positive drainage away from the structure should be provided at all times, including during construction. If positive drainage is not provided, water will pond around or below the building and excessive total and differential movements may occur. Proper surface drainage should be maintained, and landscape irrigation systems should be located and operated in a manner to minimize wetting of building foundations. After installation, the irrigation system should be pressure tested and any leaks repaired.

Foundation Subgrade Preparation:

To prepare for foundation and soil supported floor slab construction, we recommend that all topsoil, vegetation, roots, and any soft soils in the building area be stripped from the site and either properly disposed or stockpiled for later use in landscaping. Utilities should be located and rerouted as necessary.

To remediate the variable soil conditions in the surficial zone and provide a consistent subgrade for slab support, GTL recommends that a uniform layer of density-approved select fill be provided beneath the floor slab. After stripping the site, the building pad should be cut to an elevation which allows the placement of at least two (2) feet of density-approved select fill below the final subgrade elevation for the floor slab. The select fill building pad should extend at least five (5) feet beyond the edge of the building.

After stripping and over-excavating as required herein, the building area should be proof-rolled with a heavy, loaded pneumatic-tired vehicle such as a 20 to 25 ton loaded dump truck. It is recommended that all areas beneath the floor slab be proof-rolled to identify loose or soft soils. All proof-rolling and undercutting activities should be witnessed by GTL or authorized representative and should be performed during a period of dry weather. Any weak areas which yield under the proof-roll, or any areas with a tendency to pump should be mitigated. Such mitigation may include over-excavation and backfilling, reprocessing to remove moisture, modification with lime or cement admixture, or using geotextiles. In the event such mitigation is required, the geotechnical engineer should be contacted to design an appropriate procedure.

After proof-rolling but prior to placing fill, the exposed soils should be scarified and then processed to a moisture content between one (1) percentage point below and three (3) percentage points above the Standard Proctor optimum. The subgrade soils should be re-compacted to a density of at least 95 percent of the Standard Proctor (ASTM D-698) maximum dry density for a depth of at least eight (8) inches below the surface.

Select Fill:

After the subgrade has been prepared and inspected, fill placement may begin. Select fill material should be free of organic or other deleterious materials, homogeneous mixture, have a maximum particle size of three (3) inches, have a liquid limit less than 40 and plasticity index between 8 and 20, and consist of silty-clayey sands (SM-SC), low plasticity sandy clays (CL), or clayey sands (SC) as defined by the Unified Soil Classification System. In addition to the above requirements, the material should have a minimum of 30 percent retained on the No. 200 sieve. Due to the high volumes of fines passing the No. 200 sieve, it appears that the on-site surficial soils do not meet the requirement for use as select fill on this project. If a fine-grained material is used for fill, very close moisture content control will be required to achieve the recommended degree of compaction.

Fill should be placed in maximum lifts of eight (8) inches of loose materials and should be compacted within the range of one (1) percentage point below to three (3) percentage points above the optimum moisture content value and a minimum of 95 percent of the maximum density as determined by the Standard Proctor (ASTM D-698) test. If water must be added, it should be uniformly applied and thoroughly mixed into the soil by diskings or scarifying.

Each lift of compacted soil should be tested and inspected by the soils engineer or his representative prior to placement of subsequent lifts. As a guideline, it is recommended that field density tests be taken at a frequency of not less than one (1) test per 2,500 square feet of surface area per lift or a minimum of four (4) per lift for each tested area for the building.

The fill can be used to elevate the building pad so that positive drainage is provided away from the building. Where feasible, elevating the building pad with fill is generally desirable because this aids in providing positive drainage away from the floor slab and foundations and helps prevent water from collecting in the filled area.

Shallow Footings:

Perimeter footings should bear at a minimum nominal depth of 24 inches below the planned finished floor elevation or 18 inches below exterior adjacent grade, whichever is deeper. Spread footings for columns and strip footings for walls may be designed for a maximum net allowable soil bearing pressure of 2,000 psf and 1,500 psf, respectively, based on dead load plus design live load. Minimum foundation widths for column and strip footings should be 24 inches and 14 inches, respectively, even if the bearing pressures are less than the recommended values.

The factor of safety for the above bearing values is 3.0. Total settlement is estimated to be on the order of one (1) inch or less for foundation units designed in accordance with recommendations provided herein. Differential settlements are estimated to be on the order of ½ inch or less. Approximately half of this settlement is expected to occur during construction. The remaining long-term settlement of ½ inch (¼ occurring differentially) should be tolerable.

All foundation excavations should be inspected by GTL or an authorized representative prior to steel and concrete placement to assess whether the foundation materials appear consistent with the boring logs. Soft or loose soil zones encountered at the bottom of the footing excavations should be removed and the cavity should be backfilled with compacted select fill, flowable grout fill, crushed stone flexible base, concrete, or other approved material and placement control.

Floor Slab and Grade Beams:

Construction of select fill as specified herein beneath the building should result in the development of a modulus of subgrade reaction (k_s) to range between 125 and 150 pounds per cubic inch based upon empirical equations that estimate the results of a plate load test.

The length of the new slab abutting the existing foundation should be doweled into the existing slab to inhibit differential movement between the two slabs. Utilities which project through the slab on grade should be designed with either some degree of flexibility or with sleeves. Such design features will help reduce damage to utility lines if vertical movements occur.

The floor slab may be placed monolithically with the grade beams, or designed and constructed as a floating slab where an isolation joint separates the floor slab from all grade beams and columns. In the former case, a crack or hinge joint may develop in the slab parallel to the exterior grade beams. The floor system type should be selected and designed by the structural engineer after considering the advantages and disadvantages of each.

Membrane Under Slab:

The decision as to whether a synthetic membrane (polyethylene or HDPE sheeting, etc.) is required below the slab should be made by the architect and structural engineer based on planned floor coverings, proximity of groundwater, planned site grading and drainage patterns, tolerance for curling, local custom, weather conditions at the time of construction, and other pertinent considerations.

Connection to Existing Structure:

It is understood that the proposed structure will be connected to an existing building. There is a significant potential for differential movement to occur between the existing and proposed structure, particularly if the two structures are supported on different types of foundations. Where the structures are connected, the design should allow for differential movement to occur without causing structural distress. Furthermore, the location and type of existing foundations should be determined so that proposed excavations and construction do not damage or encounter the existing construction.

Seismicity:

Based on Section 1613 of the IBC-2012, a Site Class of D has been estimated for this site due to the lack of subsurface information to a depth of 100 feet. According to the USGS website for Seismic Hazard Design Parameters, the project site has a mapped 0.2 second spectral response acceleration (S_s) of 0.101 g. The project also has a mapped 1.0 second spectral response acceleration (S_1) of 0.057. The design spectral response accelerations, S_{DS} and S_{DI} , were determined to be 0.108 g and 0.091 g, respectively. Based on Tables 1613.3.5(1) and 1613.3.5(2), the site has an assigned Seismic Design Category of B for structures classified as Risk Categories I, II, and III. For structures classified as Risk Category IV, site has an assigned Seismic Design Category of C.

Construction Considerations:

Excessive movement should not occur if customary measures are taken to minimize moisture variations beneath the structure to preclude loss of shear strength of foundation soils. Proper surface drainage should be maintained, and landscape irrigation systems should be located and operated in a manner to minimize wetting of building foundations. Positive drainage away from the building should be provided at all times, including during construction. If positive drainage is not provided, water will pond around or below the building and excessive total and differential movements may occur.

Secondary Design Considerations:

The following information has been assimilated after examination of numerous problems dealing with soil strata throughout Louisiana. It is presented here for implementation by others. If these features are not incorporated, then performance of the structure may be "**at-risk**".

1. Roof drainage should be **routed via pipe or a hard surface at least 5 feet from the structure.**
2. The **depth of frost penetration** in the vicinity of the project site is estimated to be approximately six inches.
3. Pavements, sidewalks, and the general ground surface should be sloped away from the structure on all sides. Water must not be allowed to pond within 5 feet of the building.
4. Backfill for utility lines should be compacted to at least 95 percent of the standard compaction test (ASTM D-698).

5. Surficial soils of the type encountered at this site are subject to erosion. Therefore, unpaved areas should be protected from erosion by the establishment of a good vegetation cover.
6. Clayey fill has been specified for select fill to reduce the potential migration of water beneath the proposed establishment. Drainage details must focus on routing water away from the structure. Excessive water intrusion can produce undesirable latent vertical movement.
7. Landscaping elements, including irrigation systems must not be allowed to introduce excess water to the structure subgrade. Monitor irrigation controls frequently and adjust to avoid over-watering of plants positioned in close proximity to the structure.

Safety Considerations:

Prior to the commencement of construction, the owner and the contractor should make themselves aware of and become familiar with applicable local, state, and federal safety regulations, including the current Occupational Safety and Health Association (OSHA) Excavation and Trench Safety Standards. Construction site safety generally is the sole responsibility of the contractor, who shall also be solely responsible for the means, methods, and sequencing of construction operations. We are providing this information solely as a service to our client. Under no circumstances should the information provided herein be construed that GTL is assuming responsibility for construction site safety of the contractor's activities. Such responsibility is not being implied and should not be inferred.

Worker Safety - Excavations and Slopes:

After excavating, footings should be inspected and concrete placed as quickly as possible to avoid exposure of the footing bottoms to wetting and drying. If it is required that footing excavations be left open for more than one (1) day, they should be protected to reduce evaporation or entry of moisture. Adequate protection against sloughing of soil should be provided for workers and inspectors entering the footing excavations and undercut areas.

The contractor should be aware that slope height, slope inclination, or excavation depths (including utility trench excavations) should in no case exceed those specified in local, state, or federal safety regulations, e.g., OSHA Standards for Excavations, Title 29, Part 1926, successor regulations as well as other building code requirements. Such regulations are strictly enforced and, if not followed, the owner, contractor, and earthwork and utility subcontractors could be liable for substantial penalties.

Drainage:

Water should not be allowed to collect near the foundations, floor slab or pavement areas of the project either during or after construction. Undercut or excavated areas should be sloped toward a sump area to facilitate removal of any collected groundwater or surface runoff. Proper drainage should be provided by sloping the ground surface away from the structure.

Weather Considerations:

The soils encountered in the surficial zone at this site are expected to be relatively sensitive to disturbances caused by construction traffic when wet. The contractor should be aware of the importance of proper maintenance of surface drainage. Depending on weather-related ground conditions, contractor's maintenance of drainage during construction, and other factors, some difficulty may be encountered by the contractor in achieving compaction on initial lifts of fill placed on loose or soft subgrade. This will be exacerbated by wet weather, particularly if the contractor allows surface drainage to enter and pond in the excavations.

Fine-grained soils are expected to be relatively sensitive to disturbances caused by construction traffic and to changes in moisture content. During wet weather periods, increases in the moisture content of the soil can cause significant reduction in the soil strength and support characteristics. In addition, soil which becomes wet may be slow to dry and thus significantly retard the progress of grading and compaction activities. It will, therefore, be advantageous to perform earthwork and foundation construction activities during dry weather. Earthwork activities performed during cooler; wetter months may certainly offer more difficulties than if performed during warmer, drier periods.

If construction is performed during wet conditions, work platforms can be created for earthwork by mixing fly ash, hydrated lime, cement, or combinations of these additives. Quick lime may also be used in areas where dusting is of concern, if proper worker safety considerations are observed. Pumping subgrades are possible at the site and it is recommended that bid documents incorporate this possibility into the bid schedule.

The use of geotextiles and geogrids may be warranted in situations where the subgrade is very wet and highly unstable, if such use is necessary to maintain a mandatory construction schedule during wet weather.

Groundwater Control:

Due to potential variations in groundwater levels, difficulty during excavation and construction of the proposed foundation is possible. Shallow groundwater was not encountered at this site. However, it is reasonable to anticipate that groundwater conditions may vary as noted previously. It is suggested that contract documents address the need for maintaining controls to preclude water from draining into excavations. Some dewatering through shaping of work areas to shed water, and construction of temporary ditches with sumps and pumping may be necessary to remove the loose soils and allow placement of imported select fill in a dry manner. Excavated soils intended for re-use as select fill may require special methods in order to dry the soil to a suitable moisture content prior to re-placing the soil as select fill.

Protection of Work:

Subgrade areas, base courses, and lifts of fill that have been successfully moisture conditioned, processed, and compacted in lifts to the required density, successfully proof-rolled, and approved must be protected from changes in moisture and other influences. Satisfactorily completed areas may be adversely affected by prolonged exposure to dry weather, precipitation, equipment traffic, or by excavations and uncontrolled backfilling for utilities, and other disturbances rendering such areas unsatisfactory. Such areas should be reworked prior to continuing with subsequent construction.

Geotechnical Risk:

The concept of risk is an important aspect of the geotechnical evaluation. The primary reason for this is that the analytical methods used to develop geotechnical recommendations do not comprise an exact science. The analytical tools which geotechnical engineers use are generally empirical and must be used in conjunction with engineering judgment and experience. Therefore, the solutions and recommendations presented in the geotechnical evaluation should not be considered risk-free and, more importantly, are not a guarantee that the interaction between the soils and the proposed structure will perform as planned. The engineering recommendations presented in the preceding sections constitutes GTL's professional estimate of those measures that are necessary for the proposed structure to perform according to the proposed design based on the information generated and referenced during this evaluation, and GTL's experience in working with these conditions.

Limitations:

The exploration and analysis of the conditions reported herein are considered sufficient in detail and scope to form a reasonable basis for the foundation design. The recommendations submitted are based on the available soil information and preliminary design details furnished for the proposed project. Any revision of the plans for the proposed facility from those enumerated in this report should be brought to our attention so that we may determine if changes in the foundation recommendations are required. If deviations from the noted subsurface conditions are encountered during construction, GTL should be retained to determine if changes in foundation recommendations are required. If GTL is not retained to perform these functions, we will not be responsible for the performance of the structure.

The findings, recommendations, specifications, or professional advice contained herein have been made after being prepared in accordance with generally accepted professional engineering practice in the fields of foundation engineering, soil mechanics, and engineering geology. No other warranties are implied or expressed.

The scope of services did not include any environmental assessment for the presence or absence of wetlands or hazardous or toxic materials in the soil, surface water, groundwater, or air, on or below or around this site. Any statements in this report or on the boring logs regarding odors, colors, or unusual or suspicious items or conditions are strictly for the information of the client. Prior to purchase or development of this site, an environmental assessment is advisable.

The scope of services did not include a geologic investigation to address any faults, large scale subsidence, or other macro geologic features not specifically addressed in this report or the agreement between GTL and the client.

After the plans and specifications are more complete, it is recommended that the soils and foundation engineer be provided the opportunity to review the final design and specifications in order that the earthwork and foundation recommendations may be properly interpreted and implemented. At that time, it may be necessary to submit supplementary recommendations.

This report has been prepared for the exclusive use of our client for the specific application to the referenced project. GTL cannot be responsible for interpretations, opinions, or recommendations made by others based on the data contained in this report.

This report was prepared for design purposes only and may not be sufficient for purposes of preparing an accurate bid for construction. Contractors reviewing this report are advised that the discussions and recommendations contained herein were provided exclusively to and for use by the project owner.

END OF REPORT TEXT

SEE FOLLOWING APPENDIX w/BORING LOGS & TEST RESULTS

APPENDIX A

FIELD AND LABORATORY PROCEDURES

Field and Laboratory Procedures
**Multi-Purpose Building for
Bayou Chicot Elementary School**
Ville Platte, Evangeline Parish, Louisiana
Report Number 03-22-036

I. Field Operations:

Subsurface conditions were evaluated by advancing four (4) intermittent sample borings on March 1, 2022 within the project area. The boring locations were selected by the Client, and staked in the field by representatives of Geotechnical Testing Laboratory, Inc. An illustration of the approximate boring locations with respect to the area investigated is provided on the Plan of Borings provided in this report. Descriptive terms and symbols used on the logs are in accordance with the Unified Soil Classification System (USCS).

The borings were advanced using a truck-mounted rotary drilling rig. Each boring was advanced in the dry, and intermittent undisturbed samples were obtained in the following manner.

Standard penetration tests were performed in accordance with ASTM D-1586 procedures. This test is conducted by recording the number of blows required for a 140-pound hammer falling 30 inches to drive a split-spoon sampler eighteen inches into the substrata. Depths at which split-spoon samples were taken are indicated by two crossed lines in the "Samples" column on the Log of Boring. The number of blows required to drive the sampler for each 6-inch increment were recorded. The penetration resistance is the number of blows required to drive the split-spoon sampler the final 12-inches of penetration. Information related to the penetration resistance is presented under the "Field Data" heading of the Log of Boring as the Standard Penetration (Blows/Foot).

The remainder of the borings were sampled from the auger. All samples were visually examined, logged, and packaged for transport to our laboratory.

The presence of ground water was monitored during drilling operations. Initial water seepage readings are provided under "Groundwater Information" in the right hand column of the Log of Boring. Upon boring completion, water levels were allowed to rise and stabilize for several minutes prior to final water readings. These readings are found under "Groundwater Information". Soil sloughing from the walls of the boring are also recorded here as depth of cave-in.

II. Laboratory Studies:

Upon return to the laboratory, all samples were visually examined and representative samples were selected for testing. Tests were performed on selected samples recovered from the test borings to verify classification and to determine pertinent engineering properties of the substrata. Individual test and ASTM designations are provided below:

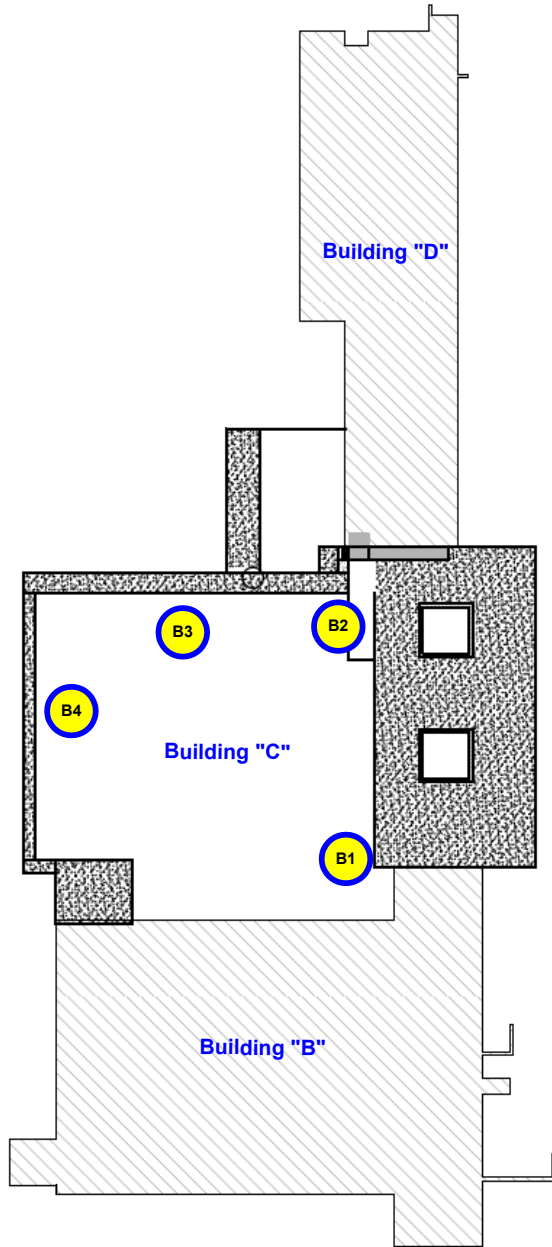
Test	ASTM Designations
Atterberg Limits	ASTM D4318
Moisture Content	ASTM D2216
Percent Minus #200	ASTM D1140

Results for soil classifications are located on the Log of Boring in their respective columns under "Laboratory Data."

Samples obtained during our field studies and not consumed by laboratory testing procedures will be retained free of charge for a period of 30 days. Arrangements for storage beyond that period of time must be made in writing to ***Geotechnical Testing Laboratory, Inc.***

APPENDIX B

PLAN OF BORINGS



This site map is intended to locate the boring relative to the general area.

PLAN OF BORINGS

PROJECT

A New Multi-Purpose Building for Bayou Chicot Elementary School, Ville Platte, Louisiana

SCALE

Not to Scale

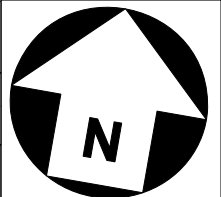
DATE

3/3/2022

FILENAME

02-22-036

Braddock Companies, LLC



APPENDIX C

BORING LOGS AND SOIL CLASSIFICATION CHART

LOG OF BORING B-1



Geotechnical Testing Laboratory, Inc.
 226 Parkwood Drive
 Alexandria, LA 71301
 Telephone: (318) 443-7429

CLIENT: **Braddock Companies, LLC**
 PROJECT: **Multi-Purpose Building for Bayou Chicot Elementary School**
 LOCATION: **Ville Platte, Evangeline Parish, Louisiana**
 FILE NO.: **03-22-036**
 DRILL DATE: **3/1/22**

FIELD DATA		LABORATORY DATA							DRILLING METHOD(S): CME 45B, 4.5" I.D. Hollow Stem Auger	
SOIL SYMBOL	DEPTH (FT)	SAMPLES	N: BLOWS/FT P: TONS/SQ FT	MOISTURE CONTENT (%)	ATTERBERG LIMITS			MINUS NO. 200 SIEVE (%)	DRY DENSITY (Lbs./Cu.Ft.)	COMPRESSIVE STRENGTH (Lb./Sq. Ft.)
					LL	PL	PI			
DRILLER: W. Hebert CHECKED BY: K. Gorsha										
GROUNDWATER INFORMATION: No Water Seepage Noted While Drilling No Water Observed Upon Completion Boring Walls Remained Open										
SURFACE ELEVATION: Not Determined										
DESCRIPTION OF STRATUM										
										Firm Yellowish Brown LEAN CLAY (CL)s w/sand
			N = 4	16	29	18	11	80		3.0'
			N = 4	20						
	5		N = 11	22	37	22	15	95		Stiff Yellowish Brown & Gray LEAN CLAY (CL)
			N = 15	24						- very stiff @ 5.5 feet
			N = 13	22	41	22	19	90		- stiff @ 7.0 feet
	10		N = 20	24						- very stiff @ 9.0 feet
			N = 5	27	38	21	17	98		- firm, reddish brown below 14.0 feet
	15									
			N = 6	28						20.0'
	20									Boring Terminated @ 20.0 Feet

GTL LOG - LOG A GNNL01.GDT - 3/4/22 05:43 - K:\GINT PROJECTS\2022 JOBS\03-22-036.GPJ

N - STANDARD PENETRATION TEST RESISTANCE
 P - POCKET PENETROMETER RESISTANCE

NOTES:
 See Plan of Borings for Location
 Stratification Is Not Exact

LOG OF BORING B-2



Geotechnical Testing Laboratory, Inc.
 226 Parkwood Drive
 Alexandria, LA 71301
 Telephone: (318) 443-7429

CLIENT: **Braddock Companies, LLC**
 PROJECT: **Multi-Purpose Building for Bayou Chicot Elementary School**
 LOCATION: **Ville Platte, Evangeline Parish, Louisiana**
 FILE NO.: **03-22-036**

DRILL DATE: **3/1/22**

FIELD DATA		LABORATORY DATA							DRILLING METHOD(S): CME 45B, 4.5" I.D. Hollow Stem Auger	
SOIL SYMBOL	DEPTH (FT)	SAMPLES	N: BLOWS/FT P: TONS/SQ FT	MOISTURE CONTENT (%)	ATTERBERG LIMITS			MINUS NO. 200 SIEVE (%)	DRY DENSITY (Lbs./Cu.Ft.)	COMPRESSIVE STRENGTH (Lb./Sq. Ft.)
					LL	PL	PI			
DRILLER: W. Hebert CHECKED BY: K. Gorsha										
GROUNDWATER INFORMATION: No Water Seepage Noted While Drilling No Water Observed Upon Completion Boring Walls Remained Open										
SURFACE ELEVATION: Not Determined										
DESCRIPTION OF STRATUM										
1.5'	Firm Yellowish Brown LEAN CLAY (CL)s w/sand									
3.5'	Loose Yellowish Brown, Poorly Graded, SAND (SP-SM)									
5	Stiff Yellowish Brown & Gray LEAN CLAY (CL)									
5.5	- very stiff @ 5.5 feet									
9.0	- stiff @ 9.0 feet									
15	- firm, reddish brown below 14.0 feet									
20.0'	Boring Terminated @ 20.0 Feet									

GTL LOG - LOG A GNNL01.GDT - 3/4/22 05:43 - K:\GINT PROJECTS\2022 JOBS\03-22-036.GPJ

N - STANDARD PENETRATION TEST RESISTANCE
 P - POCKET PENETROMETER RESISTANCE

NOTES:
 See Plan of Borings for Location
 Stratification Is Not Exact

LOG OF BORING B-3



Geotechnical Testing Laboratory, Inc.
 226 Parkwood Drive
 Alexandria, LA 71301
 Telephone: (318) 443-7429

CLIENT: **Braddock Companies, LLC**
 PROJECT: **Multi-Purpose Building for Bayou Chicot Elementary School**
 LOCATION: **Ville Platte, Evangeline Parish, Louisiana**
 FILE NO.: **03-22-036**

DRILL DATE: **3/1/22**

SOIL SYMBOL	FIELD DATA			LABORATORY DATA						DRY DENSITY (Lbs./Cu.Ft.)		COMPRESSIVE STRENGTH (Lb./Sq. Ft.)		DRILLING METHOD(S): CME 45B, 4.5" I.D. Hollow Stem Auger		
	DEPTH (FT)	SAMPLES	N: BLOWS/FT P: TONS/SQ.FT	MOISTURE CONTENT (%)	ATTERBERG LIMITS			MINUS NO. 200 SIEVE (%)	DRILLER: W. Hebert CHECKED BY: K. Gorsha							
					LL	PL	PI		GROUNDWATER INFORMATION: No Water Seepage Noted While Drilling No Water Observed Upon Completion Boring Walls Remained Open							
													SURFACE ELEVATION: Not Determined			
DESCRIPTION OF STRATUM																
1.5'	Stiff Yellowish Brown LEAN CLAY (CL)s w/sand															
3.0'	Medium Dense Yellowish Brown Silty SAND (SM)															
5	Stiff Yellowish Brown & Gray LEAN CLAY (CL)															
10	- very stiff @ 9.0 feet															
15	- firm, reddish brown below 14.0 feet															
20.0'	Boring Terminated @ 20.0 Feet															

GTL LOG - LOG A GNNL01.GDT - 3/4/22 05:43 - K:\GINT PROJECTS\2022 JOBS\03-22-036.GPJ

N - STANDARD PENETRATION TEST RESISTANCE
 P - POCKET PENETROMETER RESISTANCE

NOTES:
 See Plan of Borings for Location
 Stratification Is Not Exact

LOG OF BORING B-4



Geotechnical Testing Laboratory, Inc.
 226 Parkwood Drive
 Alexandria, LA 71301
 Telephone: (318) 443-7429

CLIENT: **Braddock Companies, LLC**
 PROJECT: **Multi-Purpose Building for Bayou Chicot Elementary School**
 LOCATION: **Ville Platte, Evangeline Parish, Louisiana**
 FILE NO.: **03-22-036**
 DRILL DATE: **3/1/22**

FIELD DATA		LABORATORY DATA							DRILLING METHOD(S): CME 45B, 4.5" I.D. Hollow Stem Auger	
SOIL SYMBOL	DEPTH (FT)	SAMPLES	N: BLOWS/FT P: TONS/SQ FT	MOISTURE CONTENT (%)	ATTERBERG LIMITS			MINUS NO. 200 SIEVE (%)	DRY DENSITY (Lbs./Cu.Ft.)	COMPRESSIVE STRENGTH (Lb./Sq. Ft.)
					LL	PL	PI			
DESCRIPTION OF STRATUM										
										Firm Reddish LEAN CLAY (CL)
										- stiff, gray @ 2.0 feet
										- yellowish brown & gray below 3.3 feet
	5		N = 4	20						- very stiff @ 5.5 feet
			N = 12	17	30	20	10	94		
			N = 14	24						
			N = 15	23	44	23	21	98		
			N = 21	22						
	10		N = 16	21	39	22	17	96		
			N = 6	28						- firm, reddish brown below 14.0 feet
	15									
			N = 8	27	37	20	17	97		
	20									20.0'
										Boring Terminated @ 20.0 Feet

GTL LOG - LOG A GNNL01.GDT - 3/4/22 05:43 - K:\GINT PROJECTS\2022 JOBS\03-22-036.GPJ

N - STANDARD PENETRATION TEST RESISTANCE
 P - POCKET PENETROMETER RESISTANCE

NOTES:
 See Plan of Borings for Location
 Stratification Is Not Exact

SOIL CLASSIFICATION CHART

MAJOR DIVISIONS			SYMBOLS		TYPICAL DESCRIPTIONS	
			GRAPH	LETTER		
<p>COARSE GRAINED SOILS</p> <p>MORE THAN 50% OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE</p>	<p>GRAVEL AND GRAVELLY SOILS</p>	<p>CLEAN GRAVELS</p> <p>(LITTLE OR NO FINES)</p>		GW	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES	
		<p>GRAVELS WITH FINES</p> <p>(APPRECIABLE AMOUNT OF FINES)</p>		GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES	
		<p>GRAVELS WITH FINES</p> <p>(APPRECIABLE AMOUNT OF FINES)</p>		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES	
		<p>GRAVELS WITH FINES</p> <p>(APPRECIABLE AMOUNT OF FINES)</p>		GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES	
	<p>SAND AND SANDY SOILS</p>	<p>CLEAN SANDS</p> <p>(LITTLE OR NO FINES)</p>		SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	
				SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES	
		<p>SANDS WITH FINES</p> <p>(APPRECIABLE AMOUNT OF FINES)</p>		SM	SILTY SANDS, SAND - SILT MIXTURES	
				SC	CLAYEY SANDS, SAND - CLAY MIXTURES	
			<p>SILTS AND CLAYS</p> <p>LIQUID LIMIT LESS THAN 50</p>		ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
					CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
<p>SILTS AND CLAYS</p> <p>LIQUID LIMIT GREATER THAN 50</p>		OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY			
		MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS			
		CH	INORGANIC CLAYS OF HIGH PLASTICITY			
		OH	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS			
<p>HIGHLY ORGANIC SOILS</p>				PT	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS	

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

DRAINAGE BASIN SCHEDULE

CB = CATCH BASIN - BY "ADS" NYLOPLAST , HANCOR OR EQUAL - INLINE CATCH BASIN									
DB = DRAIN BASIN BY "NDS" OR EQUAL : PROVIDE CAST IRON DRAIN BASIN GRATE TOPS AS SCHEDULED									
DS BASINS = 9X9 WITH BRASS TOPS - CUT TOPS TO RECEIVE DOWNSPOUT OUTLET									
CB - GRATE LOADING - DUCTILE IRON HD-10 LIGHT DUTY - HD-20 HEAVY DUTY TRAFFIC									
CB - GRATE TYPE - STANDARD , SOLID, ATRIUM (dome top)									
FF = FINISH FLOOR OF GYM = 100 = 0-0 = *0"									
BASIN TYPE & #	TOP ELEV	INVERT	INLET 1	INLET 2	INLET 3	OUTLET	BASIN SIZE	GRATE TYPE	GRATE LOAD #
CB 1	GRADE	-18"	10"			10"	12"	STANDARD	HD-20
CB 2	GRADE	24"	10"	6"	6"	10"	12"	STANDARD	HD-20
CB 3	GRADE	28.2"	10"	6"	6"	10"	12"	STANDARD	HD-20
CB 4	-3"	31.92"	10"	10" VOJ	10" VOJ	10"	12"	STANDARD	HD-20
CB 5	-3"	-36"	10"			10"	12"	STANDARD	HD-20
CB 6	-12"	-45"	12"			12"	24"	ATRIUM	HD-10
CB 7	GRADE	-60"	15"			15"	24"	SOLID	HD-10
CB 8	GRADE	-30"				12"	24"	STANDARD	HD-10
CB 9									
DB 1	-6"	-18"				4"	9x9	CAST IRON	
DB 2	-3"	20"	4"	4	6"	6"	9x9	ATRIUM	
DB 3	-6"	-18"	4"		4"	4"	9x9	CAST IRON	
DB 4	-3"	-15"				6"	9x9	ATRIUM	
DS BASINS	SLOPE PIPE TO MEET MAIN DRAINAGE LINE					4"	9x9	BRASS	